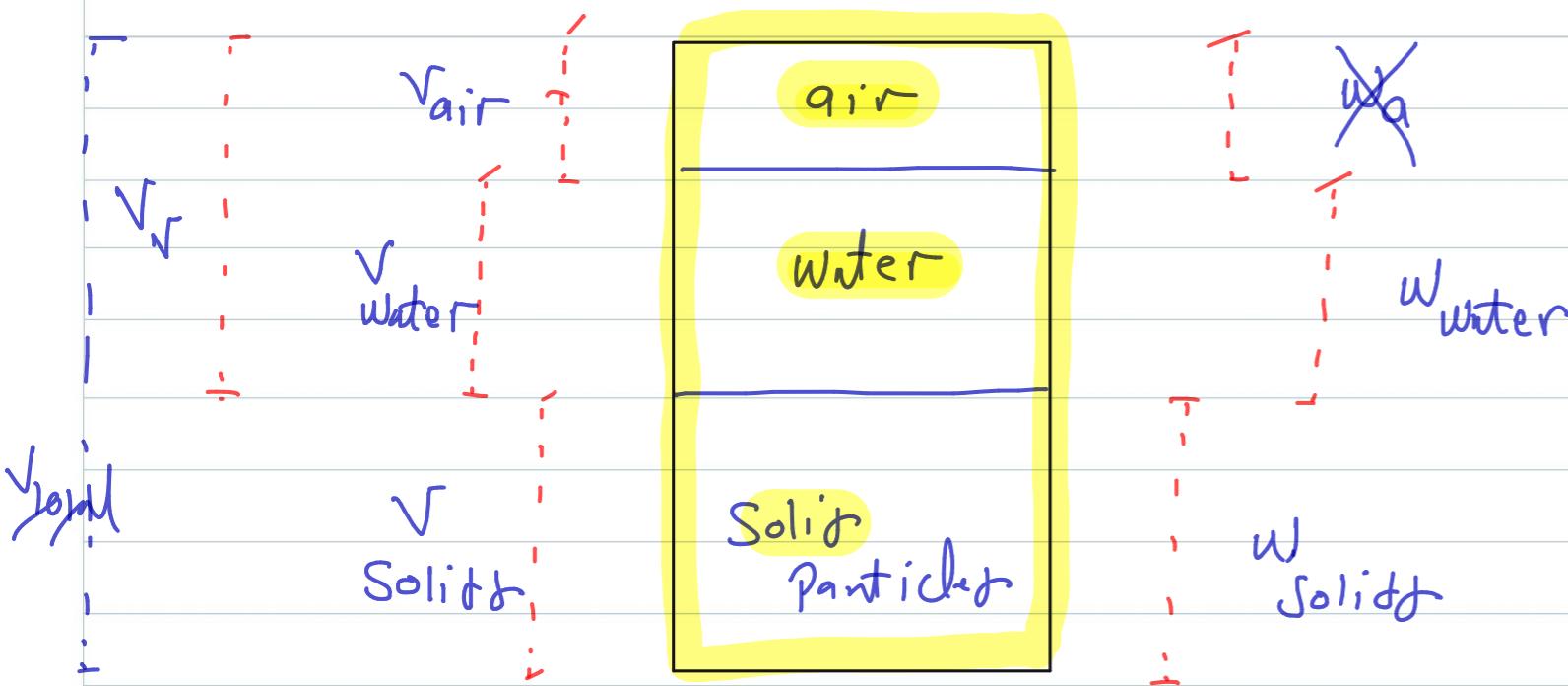


Geotechnical



$$\begin{aligned} V_{\text{total}} &= V_s + V_v \\ &= V_s + V_w + V_a \end{aligned}$$

$$w_{\text{total}} = w_s + w_w$$

$$\Rightarrow \text{Water Content } (w) = \frac{w_w}{w_s}$$

= Moisture Content

Dry Soil $w = 0$ $\therefore w > 1$

$$\text{Void Ratio } e = \frac{\frac{V_w + V_a}{V_v}}{\frac{V_s}{V_v}} \quad \leftarrow e \neq 0$$

$e > 1$

$$\text{Porosity } (n) = \frac{V_v}{V_t} \quad \leftarrow n \neq 0$$

$n \geq 1$

$$* n = \frac{e}{1+e}$$

$$* e = \frac{n}{1-n}$$

$$* \text{Degree of Saturation } (S_r) = \frac{V_w}{V_v}$$

$S_r = 1$ Fully Saturated

$S_r = 0$ Dry Soil

$0 < S_r < 1$ Partially Saturated

$$\times \gamma_f$$

$$\gamma_{\text{sat}}$$

$$\gamma_{\text{sub}}$$

$$\rightarrow \gamma = \frac{\text{Weight}}{\text{Volume}}$$

$$\Rightarrow \gamma_{\text{bulk}} = \gamma_{\text{moist}}$$

$$= \gamma_{\text{wet}} = \gamma_{\text{partially}}$$

$$* G \cdot S \Rightarrow \text{Specific Gravity} = \frac{\gamma_s}{\gamma_w}$$

* $w \cdot G_s = e \cdot S_r$

$$2.6 < G_s < 2.75$$

$$\Rightarrow \gamma_{bulk} = \gamma_w G_s \frac{1+w}{1+e}$$

Dry Condition
 $w=0$ & $V_v = V_a$

$$\gamma_{dry} = \frac{\gamma_w G_s}{1+e}$$

$$\gamma_{dry} = \frac{\gamma_{bulk}}{1+w}$$

$$= \frac{w_s}{V_s + V_v}$$

air

Fully saturated

$$S_r = 1$$

$$w = \frac{e}{G_s}$$

$$\gamma_{sat} = \frac{\gamma_w (G_s + e)}{1+e}$$

or

$$= \frac{w_s + w_w}{V_s + V_w}$$

Water

$$V_v = V_w$$

$$\begin{aligned}\gamma_{sub} &= \gamma' \\ &= \gamma_{sat} - \gamma_w\end{aligned}$$

Effective
Unit
Weight

14 15 16 17 18 19 20 21 22

$\frac{KN}{m^3}$

$$\gamma_w = 9.81 \frac{KN}{m^3}$$

Numerical Example 1:

A soil sample has a void ratio, water content, and specific gravity of 55%, 17%, and 2.65, respectively. Determine the soil degree of saturation, porosity, natural unit weight, submerged unit weight, and dry unit weight.

Solution

$$e = 0.55$$

$$w = 0.17$$

$$G_s = 2.65$$

$$\Rightarrow w \cdot G_s = e \cdot S_r \Rightarrow S_r = \frac{w G_s}{e} = \frac{0.17 * 2.65}{0.55} = 0.82 = 82\%$$

$$\Rightarrow n = \frac{e}{1+e} = \frac{0.55}{1+0.55} = 0.35 = 35\%$$

$$\Rightarrow \gamma = \gamma_w G_s \frac{1+w}{1+e} = 9.81 * 2.65 \frac{1+0.17}{1+0.55} = 19.62 \text{ KN/m}^3$$

$$\Rightarrow \gamma_{sat} = \gamma_w \frac{G_s + e}{1+e} = 9.81 * \frac{2.65 + 0.55}{1+0.55} = 20.25 \text{ KN/m}^3$$

$$\Rightarrow \gamma_{sub} = \gamma_{sat} - \gamma_w = 20.25 - 9.81 = 10.44 \text{ KN/m}^3$$

$$\Rightarrow \gamma_{dry} = \frac{\gamma_w G_s}{1+e} = \frac{9.81 * 2.65}{1+0.55} = 16.77 \text{ KN/m}^3$$

1- A saturated soil has a water content of 40%. Determine the saturated, dry, and submerged unit weights of this soil ($G_s = 2.71$). ✓ ✓

$$S_r = 1$$

$$\omega = 0.4$$

$$G_s = 2.71$$

$$\omega G_s = e \cdot S_r$$

$$e = \frac{\omega G_s}{S_r} = \frac{0.4 * 2.71}{1} = 1.084$$

$$\gamma_{sat} = \frac{\gamma_w (G_s + e)}{1 + e}$$

$$= \frac{9.81 (2.71 + 1.084)}{1 + 1.084} = 17.86 \text{ kN/m}^3$$

$$\gamma_{sub} = \gamma - \gamma_w$$

$$= 17.86 - 9.81 = 8.05 \text{ kN/m}^3$$

$$\gamma_{dry} = \frac{\gamma_w G_s}{1 + e}$$

$$= \frac{9.81 * 2.71}{1 + 1.084} = 12.76 \text{ kN/m}^3$$

- 2- The dry unit weight of sand with a porosity of 0.387 is 16 kN/m^3 . Find the void ratio of the sand and unit weight of its solids.

$$e = \frac{n}{1-n} = \frac{0.387}{1-0.387} = 0.631$$

$$\gamma_{\text{dry}} = \frac{\gamma_w G_s}{1+e}$$

$$16 = \frac{9.81 G_s}{1+0.631} \quad | \quad G_s = 2.66$$

$$G_s = \frac{\gamma_s}{\gamma_w}$$

$$\gamma_s = G_s \gamma_w = 2.66 * 9.81$$

$$= 26.096 \text{ kN/m}^3$$

$$S_r = 1$$

3- A saturated soil sample has a volume of 190 cm^3 and a weight of 3.43 N . Determine its void ratio, porosity, water content and unit weight ($G_s = 2.7$).

$$V = 190 \text{ cm}^3$$

$$w = 3.43$$

$$\frac{G_s}{G_u} = 2.7$$

$$* \gamma_{Jat} = \frac{w}{V} = \frac{3.43/1000}{190 \times 10^{-6}} = 18.05 \text{ KN/m}^3$$

$$\gamma_{Jat} = \gamma_w \left(\frac{G_s + e}{1+e} \right)$$

$$18.05 = 9.81 \left(\frac{2.7 + e}{1+e} \right)$$

4

$$e = 1.023 \quad \text{Void Ratio}$$

$$* n = \frac{e}{1+e} = \frac{1.023}{1+1.023} = 0.506$$

$$\omega \cdot G_s = e \cdot S_r$$

$$\omega = \frac{e \cdot S_r}{G_s}$$

$$= \frac{1.023 \times 1}{2.7} = 0.379$$

$$* \gamma = 18.05 \text{ KN/m}^3$$

$$* \gamma = \gamma_w G_s \frac{1+w}{1+e}$$

$$= 9.81 \times 2.7 \times \frac{1+0.379}{1+1.023}$$

$$= \checkmark \text{ KN/m}^3$$

Numerical Example 2:

Aggregates from a material storage site are required for construction of roadway embankment. The porosity of the aggregates at the storage site is 80%. Calculate the volume of aggregates that should be taken from the storage site to construct a 7.6m × 305m × 2.1m embankment of soil compacted to porosity of 20%.

Solution

$$n_1 = 0.8$$

$$\gamma = ??$$

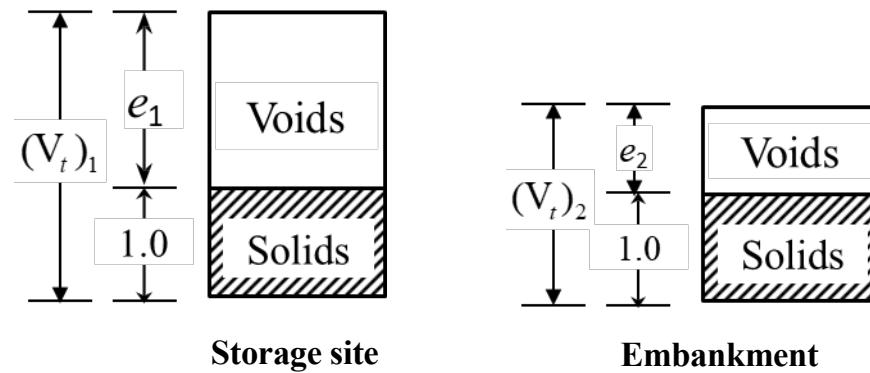
$$n_2 = 0.2$$

$$\text{Volume} = 7.6 \times 305 \\ 2 \quad * 2.1$$

Given: – Storage site, $n_1 = 0.8$

– embankment, $n_2 = 0.2$, dimensions = $7.6 \times 305 \times 2.1$ m

Required: $(V_t)_1$

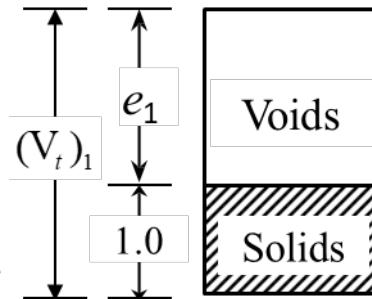


$$e = \frac{n}{1-n}$$

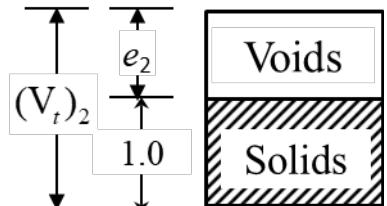
$$e_1 = \frac{n_1}{1-n_1} = \frac{0.8}{1-0.8} = 4$$

$$e_2 = \frac{n_2}{1-n_2} = \frac{0.2}{1-0.2} = 0.25$$

$\frac{(V_t)_1}{(V_t)_2} = \frac{1+e_1}{1+e_2}$



Storage site



Embankment

$$\Rightarrow (V_t)_1 = \left(\frac{1+e_1}{1+e_2} \right) (V_t)_2$$

$$= \frac{1+4}{1+0.25} (7.6 + 30.5 + 2.1)$$

$$= 194.71 \text{ m}^3$$



